

**PRESCRIPTIVE TABLES**  
FOR  
**SELECTION OF SUPPORT ELEMENTS FOR  
BEAMS, GIRDERS, AND HEADERS**

**70 PSF ROOF SNOW LOAD**

FOR USE WITH  
**2018 NORTH CAROLINA RESIDENTIAL CODE**

**FEBRUARY 2018**



## FOREWORD

The following tables are updated versions of tables that were originally issued in September of 2003. The tables are based on, and intended for use with, the 2018 North Carolina Residential Code (2015 International Residential Code with North Carolina amendments). The following tables are intended for use with structures subject to a maximum roof snow load of 70 psf. Table packages are also available for roof snow loads of 20 psf, 30 psf, and 50 psf.

The tables are intended as a practical tool to assist contractors in:

1. Selecting footing widths.
2. Determining the number of wood studs required to support the end reactions of beams, girders, and/or headers.

The values presented in the tables were derived from the minimum design criteria, maximum span conditions, and allowable loads published in the 2018 North Carolina Residential Code. The section(s) of the 2018 North Carolina Residential Code used in the development of the tabulated values is noted on each table.

Structural elements that exceed the prescriptive limitations of the 2018 North Carolina Residential Code and/or these tables, must be designed in accordance with accepted engineering practice by a registered design professional.

### Example Problem:

A 4.5-inch-thick header (3-2x plies) with an end reaction of 6400 pounds is located in the first story exterior wall of a 28-foot-wide, two-story house with center bearing floors and exterior brick veneer. The load bearing value of the soil is 2000 psf and the roof snow load is 70 psf. Determine the quantity of jack studs and the minimum footing width required to support the reaction at each end of the header.

### Solution:

Table 2.1 indicates that 4 (four) jack studs are required at each end of the header (Maximum End Reaction = 7040 pounds with a 4.5-inch-thick header). Table 1.4 indicates that a 32-inch-wide by 10-inch-thick continuous footing is required to support the header reactions (Maximum Column Load = 7337 pounds; refer to Figure 4, *Roof, Ceiling and Two Center-Bearing Floors*). Note that Table 1.4 is based on a 36-foot-wide house.

### Example Problem:

A header with a span of 8 feet is located in the first story exterior wall of a 34-foot-wide, three-story house with center bearing floors and wood exterior cladding. The load bearing value of the soil is 2000 psf and the roof snow load is 70 psf. Determine the quantity of jack studs and the minimum footing width required to support the reaction at each end of the header.

### Solution:

Table 2.2 indicates that the header has an end reaction of 10,096 pounds (refer to Figure 13, *Roof, Ceiling, and Two Center-Bearing Floors*) and that 4 (four) jack studs are required for a 6-inch-thick header. Table 1.1 indicates that a 36-inch-wide by 14-inch-thick continuous footing is required to support the header reactions (Maximum Column Load = 12,739 pounds; refer to Figure 6, *Roof, Ceiling, and Three Center-Bearing Floors*). Note that both Table 2.2 and Table 1.1 are based on a 36-foot-wide house.

**Example Problem:**

An interior beam is required to support a uniform load of 600 plf over a span of 8 feet. Select an appropriate wood beam for the load and span specified and determine the quantity of jack studs required to support each end of the beam.

**Solution:**

Table 3.1 indicates that a 3-ply 2x10 spruce-pine-fir beam is required to support a load of 600 plf over a span of 8 feet (Allowable Load = 643 plf). Table 3.1 also indicates, for a 3-ply 2x10 with a uniform load of 643 plf and a span of 8 feet, an end reaction of 2572 pounds. Using this reaction, Table 2.1 indicates that 2 jack studs are required to support each end of the beam (Maximum End Reaction = 3520 pounds with a 4.5-inch-thick header).

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## **FOOTING TABLES**

**TABLE 1.1: MAXIMUM COLUMN LOAD ON CONTINUOUS CAST-IN-PLACE CONCRETE FOOTING AT EXTERIOR WALL  
LIGHT-FRAME CONSTRUCTION / ROOF SNOW LOAD = 70 PSF  
(Reference: 2018 North Carolina Residential Code Section 403.1.1)**

LOAD BEARING VALUE OF SOIL (PSF)			2000								
FOOTING WIDTH, W (IN.)			16	20	24	28	32	36	40	44	48
FOUNDATION WALL SUPPORTING	FOOTING THICKNESS, T (IN.)	WALL LOAD ON FOOTING (PLF)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)
ROOF AND CEILING OVER SLAB-ON-GRADE (FIGURE 1)	8	1850	3369	6681	9994						
	10	1850	3373	6834	10295	13756					
	12	1850	3354	6958	10563	14167	17771				
	14	1850	3314	7055	10797	14538	18280	22021			
	16	1850	3251	7124	10998	14871	18744	22618	26491		
	18	1850	3166	7165	11165	15165	19164	23164	27163	31163	
20	1850	3059	7179	11299	15419	19540	23660	27780	31901	36021	
ROOF, CEILING AND ONE CENTER-BEARING FLOOR (FIGURE 2)	8	2300	856	4169	7482						
	10	2300	710	4171	7633	11094					
	12	2300	542	4146	7750	11354	14958				
	14	2300	351	4093	7834	11576	15317	19059			
	16	2300	138	4012	7885	11758	15632	19505	23378		
	18	2300	-	3903	7903	11902	15902	19901	23901	27900	
20	2300	-	3766	7887	12007	16127	20248	24368	28488	32608	
ROOF, CEILING AND ONE CLEAR-SPAN FLOOR (FIGURE 3)	8	2750	-	1656	4969						
	10	2750	-	1509	4970	8431					
	12	2750	-	1333	4938	8542	12146				
	14	2750	-	1130	4872	8613	12355	16096			
	16	2750	-	899	4773	8646	12519	16393	20266		
	18	2750	-	640	4640	8640	12639	16639	20638	24638	
20	2750	-	354	4474	8594	12715	16835	20955	25076	29196	
ROOF, CEILING AND TWO CENTER-BEARING FLOORS (FIGURE 4)	8	2780	-	1489	4802						
	10	2780	-	1331	4793	8254					
	12	2780	-	1146	4750	8354	11958				
	14	2780	-	933	4674	8416	12157	15899			
	16	2780	-	692	4565	8438	12312	16185	20058		
	18	2780	-	423	4423	8422	12422	16421	20421	24420	
20	2780	-	126	4247	8367	12487	16608	20728	24848	28968	
ROOF, CEILING AND TWO CLEAR-SPAN FLOORS (FIGURE 5)	8	3590	-	-	279						
	10	3590	-	-	-	3461					
	12	3590	-	-	-	3292	6896				
	14	3590	-	-	-	3083	6825	10566			
	16	3590	-	-	-	2836	6709	10583	14456		
	18	3590	-	-	-	2550	6549	10549	14548	18548	
20	3590	-	-	-	2224	6345	10465	14585	18706	22826	
ROOF, CEILING AND THREE CENTER-BEARING FLOORS (FIGURE 6)	8	3260	-	-	2122						
	10	3260	-	-	1953	5414					
	12	3260	-	-	1750	5354	8958				
	14	3260	-	-	1514	5256	8997	12739			
	16	3260	-	-	1245	5118	8992	12865	16738		
	18	3260	-	-	943	4942	8942	12941	16941	20940	
20	3260	-	-	607	4727	8847	12968	17088	21208	25328	
ROOF, CEILING AND THREE CLEAR-SPAN FLOORS (FIGURE 7)	8	4430	-	-	-						
	10	4430	-	-	-						
	12	4430	-	-	-		1646				
	14	4430	-	-	-		1295	5036			
	16	4430	-	-	-		899	4773	8646		
	18	4430	-	-	-		459	4459	8458	12458	
20	4430	-	-	-		-	4095	8215	12336	16456	

**NOTES:**

1. Refer to Table Notes on page 1.7 for additional information.
2. Refer to Figures 1 through 9 on pages 1.10 and 1.11 for additional information.

**TABLE 1.2: MAXIMUM COLUMN LOAD ON CONTINUOUS CAST-IN-PLACE CONCRETE FOOTING AT EXTERIOR WALL  
LIGHT-FRAME CONSTRUCTION / ROOF SNOW LOAD = 70 PSF  
(Reference: 2018 North Carolina Residential Code Section 403.1.1)**

LOAD BEARING VALUE OF SOIL (PSF)			3000								
FOOTING WIDTH, W (IN.)			16	20	24	28	32	36	40	44	48
FOUNDATION WALL SUPPORTING	FOOTING THICKNESS, T (IN.)	WALL LOAD ON FOOTING (PLF)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)
ROOF AND CEILING OVER SLAB-ON-GRADE (FIGURE 1)	8	1850	10813	15987	21161						
	10	1850	11261	16695	22128	27562					
	12	1850	11688	17375	23063	28750	34438				
	14	1850	12091	18027	23963	29899	35835	41771			
	16	1850	12473	18652	24831	31010	37189	43368	49546		
	18	1850	12833	19249	25665	32081	38498	44914	51330	57746	
	20	1850	13170	19818	26466	33114	39762	46410	53058	59706	66354
ROOF, CEILING AND ONE CENTER-BEARING FLOOR (FIGURE 2)	8	2300	8301	13474	18648						
	10	2300	8599	14032	19466	24899					
	12	2300	8875	14563	20250	25938	31625				
	14	2300	9129	15065	21001	26937	32873	38809			
	16	2300	9361	15539	21718	27897	34076	40255	46434		
	18	2300	9570	15986	22403	28819	35235	41651	48068	54484	
	20	2300	9757	16405	23053	29701	36349	42998	49646	56294	62942
ROOF, CEILING AND ONE CLEAR-SPAN FLOOR (FIGURE 3)	8	2750	5788	10962	16136						
	10	2750	5936	11370	16803	22237					
	12	2750	6063	11750	17438	23125	28813				
	14	2750	6166	12102	18038	23974	29910	35846			
	16	2750	6248	12427	18606	24785	30964	37143	43321		
	18	2750	6308	12724	19140	25556	31973	38389	44805	51221	
	20	2750	6345	12993	19641	26289	32937	39585	46233	52881	59529
ROOF, CEILING AND TWO CENTER-BEARING FLOORS (FIGURE 4)	8	2780	5621	10794	15968						
	10	2780	5759	11192	16626	22059					
	12	2780	5875	11563	17250	22938	28625				
	14	2780	5969	11905	17841	23777	29713	35649			
	16	2780	6041	12219	18398	24577	30756	36935	43114		
	18	2780	6090	12506	18923	25339	31755	38171	44588	51004	
	20	2780	6117	12765	19413	26061	32709	39358	46006	52654	59302
ROOF, CEILING AND TWO CLEAR-SPAN FLOORS (FIGURE 5)	8	3590	1098	6272	11446						
	10	3590	966	6400	11833	17267					
	12	3590	813	6500	12188	17875	23563				
	14	3590	636	6572	12508	18444	24380	30316			
	16	3590	438	6617	12796	18975	25154	31333	37511		
	18	3590	218	6634	13050	19466	25883	32299	38715	45131	
	20	3590	-	6623	13271	19919	26567	33215	39863	46511	53159
ROOF, CEILING AND THREE CENTER-BEARING FLOORS (FIGURE 6)	8	3260	2941	8114	13288						
	10	3260	2919	8352	13786	19219					
	12	3260	2875	8563	14250	19938	25625				
	14	3260	2809	8745	14681	20617	26553	32489			
	16	3260	2721	8899	15078	21257	27436	33615	39794		
	18	3260	2610	9026	15443	21859	28275	34691	41108	47524	
	20	3260	2477	9125	15773	22421	29069	35718	42366	49014	55662
ROOF, CEILING AND THREE CLEAR-SPAN FLOORS (FIGURE 7)	8	4430	-	1582	6756						
	10	4430	-	1430	6863	12297					
	12	4430	-	1250	6938	12625	18313				
	14	4430	-	1042	6978	12914	18850	24786			
	16	4430	-	807	6986	13165	19344	25523	31701		
	18	4430	-	544	6960	13376	19793	26209	32625	39041	
	20	4430	-	253	6901	13549	20197	26845	33493	40141	46789

**NOTES:**

1. Refer to Table Notes on page 1.7 for additional information.
2. Refer to Figures 1 through 9 on pages 1.10 and 1.11 for additional information.

**TABLE 1.3: MAXIMUM COLUMN LOAD ON CONTINUOUS CAST-IN-PLACE CONCRETE FOOTING AT EXTERIOR WALL  
LIGHT-FRAME CONSTRUCTION / ROOF SNOW LOAD = 70 PSF  
(Reference: 2018 North Carolina Residential Code Section 403.1.1)**

LOAD BEARING VALUE OF SOIL (PSF)			4000								
FOOTING WIDTH, W (IN.)			16	20	24	28	32	36	40	44	48
FOUNDATION WALL SUPPORTING	FOOTING THICKNESS, T (IN.)	WALL LOAD ON FOOTING (PLF)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)
ROOF AND CEILING OVER SLAB-ON-GRADE (FIGURE 1)	8	1850	18258	25293	32328						
	10	1850	19150	26556	33962	41367					
	12	1850	20021	27792	35563	43333	51104				
	14	1850	20869	29000	37130	45260	53391	61521			
	16	1850	21695	30180	38664	47149	55633	64118	72602		
	18	1850	22499	31332	40165	48998	57831	66664	75497	84330	
ROOF, CEILING AND ONE CENTER-BEARING FLOOR (FIGURE 2)	8	2300	15745	22780	29815						
	10	2300	16488	23893	31299	38705					
	12	2300	17208	24979	32750	40521	48292				
	14	2300	17907	26037	34168	42298	50428	58559			
	16	2300	18583	27067	35552	44036	52521	61005	69489		
	18	2300	19237	28070	36903	45735	54568	63401	72234	81067	
ROOF, CEILING AND ONE CLEAR-SPAN FLOOR (FIGURE 3)	8	2750	13233	20268	27303						
	10	2750	13825	21231	28637	36042					
	12	2750	14396	22167	29938	37708	45479				
	14	2750	14944	23075	31205	39335	47466	55596			
	16	2750	15470	23955	32439	40924	49408	57893	66377		
	18	2750	15974	24807	33640	42473	51306	60139	68972	77805	
ROOF, CEILING AND TWO CENTER-BEARING FLOORS (FIGURE 4)	8	2780	13065	20100	27135						
	10	2780	13648	21053	28459	35865					
	12	2780	14208	21979	29750	37521	45292				
	14	2780	14747	22877	31008	39138	47268	55399			
	16	2780	15263	23747	32232	40716	49201	57685	66169		
	18	2780	15757	24590	33423	42255	51088	59921	68754	77587	
ROOF, CEILING AND TWO CLEAR-SPAN FLOORS (FIGURE 5)	8	3590	8543	15578	22613						
	10	3590	8855	16261	23667	31072					
	12	3590	9146	16917	24688	32458	40229				
	14	3590	9414	17545	25675	33805	41936	50066			
	16	3590	9660	18145	26629	35114	43598	52083	60567		
	18	3590	9884	18717	27550	36383	45216	54049	62882	71715	
ROOF, CEILING AND THREE CENTER-BEARING FLOORS (FIGURE 6)	8	3260	10385	17420	24455						
	10	3260	10808	18213	25619	33025					
	12	3260	11208	18979	26750	34521	42292				
	14	3260	11587	19717	27848	35978	44108	52239			
	16	3260	11943	20427	28912	37396	45881	54365	62849		
	18	3260	12277	21110	29943	38775	47608	56441	65274	74107	
ROOF, CEILING AND THREE CLEAR-SPAN FLOORS (FIGURE 7)	8	4430	3853	10888	17923						
	10	4430	3885	11291	18697	26102					
	12	4430	3896	11667	19438	27208	34979				
	14	4430	3884	12015	20145	28275	36406	44536			
	16	4430	3850	12335	20819	29304	37788	46273	54757		
	18	4430	3794	12627	21460	30293	39126	47959	56792	65625	
	20	4430	3716	12892	22068	31243	40419	49595	58771	67947	77123

**NOTES:**

1. Refer to Table Notes on page 1.7 for additional information.
2. Refer to Figures 1 through 9 on pages 1.10 and 1.11 for additional information.



**TABLE 1.4: MAXIMUM COLUMN LOAD ON CONTINUOUS CAST-IN-PLACE CONCRETE FOOTING AT EXTERIOR WALL  
LIGHT-FRAME CONSTRUCTION WITH BRICK VENEER / ROOF SNOW LOAD = 70 PSF  
(Reference: 2018 North Carolina Residential Code Section 403.1.1)**

LOAD BEARING VALUE OF SOIL (PSF)			2000								
FOOTING WIDTH, W (IN.)			16	20	24	28	32	36	40	44	48
FOUNDATION WALL SUPPORTING	FOOTING THICKNESS, T (IN.)	WALL LOAD ON FOOTING (PLF)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)
ROOF AND CEILING OVER SLAB-ON-GRADE (FIGURE 1)	8	2270	1247	4560	7873	11185					
	10	2270	1124	4585	8047	11508	14969				
	12	2270	979	4583	8188	11792	15396	19000			
	14	2270	812	4553	8295	12037	15778	19520	23261		
	16	2270	622	4496	8369	12243	16116	19989	23863	27736	
	18	2270	411	4410	8410	12410	16409	20409	24408	28408	32408
20	2270	177	4297	8418	12538	16658	20778	24899	29019	33139	
ROOF, CEILING AND ONE CENTER-BEARING FLOOR (FIGURE 2)	8	2720	-	2047	5360	8673					
	10	2720	-	1923	5384	8845	12307				
	12	2720	-	1771	5375	8979	12583	16188			
	14	2720	-	1591	5333	9074	12816	16557	20299		
	16	2720	-	1383	5257	9130	13003	16877	20750	24623	
	18	2720	-	1148	5148	9147	13147	17146	21146	25145	29145
20	2720	-	885	5005	9125	13246	17366	21486	25606	29727	
ROOF, CEILING AND ONE CLEAR-SPAN FLOOR (FIGURE 3)	8	3170	-	-	2848	6160					
	10	3170	-	-	2722	6183	9644				
	12	3170	-	-	2563	6167	9771	13375			
	14	3170	-	-	2370	6112	9853	13595	17336		
	16	3170	-	-	2144	6018	9891	13764	17638	21511	
	18	3170	-	-	1885	5885	9884	13884	17883	21883	25883
20	3170	-	-	1593	5713	9833	13953	18074	22194	26314	
ROOF, CEILING AND TWO CENTER-BEARING FLOORS (FIGURE 4)	8	3560	-	-	670	3983					
	10	3560	-	-	414	3875	7337				
	12	3560	-	-	125	3729	7333	10938			
	14	3560	-	-	-	3544	7286	11027	14769		
	16	3560	-	-	-	3320	7193	11067	14940	18813	
	18	3560	-	-	-	3057	7057	11056	15056	19055	23055
20	3560	-	-	-	2755	6876	10996	15116	19236	23357	
ROOF, CEILING AND TWO CLEAR-SPAN FLOORS (FIGURE 5)	8	4370	-	-	-	-					
	10	4370	-	-	-	-	2544				
	12	4370	-	-	-	-	2271	5875			
	14	4370	-	-	-	-	1953	5695	9436		
	16	4370	-	-	-	-	1591	5464	9338	13211	
	18	4370	-	-	-	-	1184	5184	9183	13183	17183
20	4370	-	-	-	-	733	4853	8974	13094	17214	
ROOF, CEILING AND THREE CENTER-BEARING FLOORS (FIGURE 6)	8	4400	-	-	-	-					
	10	4400	-	-	-	-	2367				
	12	4400	-	-	-	-	2083	5688			
	14	4400	-	-	-	-	1756	5497	9239		
	16	4400	-	-	-	-	1383	5257	9130	13003	
	18	4400	-	-	-	-	967	4966	8966	12965	16965
20	4400	-	-	-	-	506	4626	8746	12866	16987	
ROOF, CEILING AND THREE CLEAR-SPAN FLOORS (FIGURE 7)	8	5570	-	-	-	-					
	10	5570	-	-	-	-					
	12	5570	-	-	-	-					
	14	5570	-	-	-	-			1536		
	16	5570	-	-	-	-			1038	4911	
	18	5570	-	-	-	-			483	4483	8483
20	5570	-	-	-	-				3994	8114	

**NOTES:**

1. Refer to Table Notes on page 1.7 for additional information.
2. Refer to Figures 1 through 9 on pages 1.10 and 1.11 for additional information.

**TABLE 1.5: MAXIMUM COLUMN LOAD ON CONTINUOUS CAST-IN-PLACE CONCRETE FOOTING AT EXTERIOR WALL  
LIGHT-FRAME CONSTRUCTION WITH BRICK VENEER / ROOF SNOW LOAD = 70 PSF  
(Reference: 2018 North Carolina Residential Code Section 403.1.1)**

LOAD BEARING VALUE OF SOIL (PSF)			3000								
FOOTING WIDTH, W (IN.)			16	20	24	28	32	36	40	44	48
FOUNDATION WALL SUPPORTING	FOOTING THICKNESS, T (IN.)	WALL LOAD ON FOOTING (PLF)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)
ROOF AND CEILING OVER SLAB-ON-GRADE (FIGURE 1)	8	2270	8691	13865	19039	24213					
	10	2270	9013	14447	19880	25313	30747				
	12	2270	9313	15000	20688	26375	32063	37750			
	14	2270	9590	15526	21462	27398	33334	39270	45206		
	16	2270	9845	16024	22203	28381	34560	40739	46918	53097	
	18	2270	10078	16494	22910	29326	35743	42159	48575	54991	61408
	20	2270	10288	16936	23584	30232	36880	43528	50176	56824	63473
ROOF, CEILING AND ONE CENTER-BEARING FLOOR (FIGURE 2)	8	2720	6179	11353	16527	21701					
	10	2720	6351	11784	17218	22651	28084				
	12	2720	6500	12188	17875	23563	29250	34938			
	14	2720	6627	12563	18499	24435	30371	36307	42243		
	16	2720	6732	12911	19090	25269	31448	37627	43806	49984	
	18	2720	6815	13231	19648	26064	32480	38896	45313	51729	58145
	20	2720	6876	13524	20172	26820	33468	40116	46764	53412	60060
ROOF, CEILING AND ONE CLEAR-SPAN FLOOR (FIGURE 3)	8	3170	3666	8840	14014	19188					
	10	3170	3688	9122	14555	19988	25422				
	12	3170	3688	9375	15063	20750	26438	32125			
	14	3170	3665	9601	15537	21473	27409	33345	39281		
	16	3170	3620	9799	15978	22156	28335	34514	40693	46872	
	18	3170	3553	9969	16385	22801	29218	35634	42050	48466	54883
	20	3170	3463	10111	16759	23407	30055	36703	43351	49999	56648
ROOF, CEILING AND TWO CENTER-BEARING FLOORS (FIGURE 4)	8	3560	1489	6663	11837	17011					
	10	3560	1381	6814	12248	17681	23114				
	12	3560	1250	6938	12625	18313	24000	29688			
	14	3560	1097	7033	12969	18905	24841	30777	36713		
	16	3560	922	7101	13280	19459	25638	31817	37996	44174	
	18	3560	725	7141	13558	19974	26390	32806	39223	45639	52055
	20	3560	506	7154	13802	20450	27098	33746	40394	47042	53690
ROOF, CEILING AND TWO CLEAR-SPAN FLOORS (FIGURE 5)	8	4370	-	2140	7314	12488					
	10	4370	-	2022	7455	12888	18322				
	12	4370	-	1875	7563	13250	18938	24625			
	14	4370	-	1701	7637	13573	19509	25445	31381		
	16	4370	-	1499	7678	13856	20035	26214	32393	38572	
	18	4370	-	1269	7685	14101	20518	26934	33350	39766	46183
	20	4370	-	1011	7659	14307	20955	27603	34251	40899	47548
ROOF, CEILING AND THREE CENTER-BEARING FLOORS (FIGURE 6)	8	4400	-	1973	7147	12321					
	10	4400	-	1844	7278	12711	18144				
	12	4400	-	1688	7375	13063	18750	24438			
	14	4400	-	1503	7439	13375	19311	25247	31183		
	16	4400	-	1291	7470	13649	19828	26007	32186	38364	
	18	4400	-	1051	7468	13884	20300	26716	33133	39549	45965
	20	4400	-	784	7432	14080	20728	27376	34024	40672	47320
ROOF, CEILING AND THREE CLEAR-SPAN FLOORS (FIGURE 7)	8	5570	-	-	614	5788					
	10	5570	-	-	355	5788	11222				
	12	5570	-	-	63	5750	11438	17125			
	14	5570	-	-	-	5673	11609	17545	23481		
	16	5570	-	-	-	5556	11735	17914	24093	30272	
	18	5570	-	-	-	5401	11818	18234	24650	31066	37483
	20	5570	-	-	-	5207	11855	18503	25151	31799	38448

**NOTES:**

1. Refer to Table Notes on page 1.7 for additional information.
2. Refer to Figures 1 through 9 on pages 1.10 and 1.11 for additional information.

**TABLE 1.6: MAXIMUM COLUMN LOAD ON CONTINUOUS CAST-IN-PLACE CONCRETE FOOTING AT EXTERIOR WALL  
LIGHT-FRAME CONSTRUCTION WITH BRICK VENEER / ROOF SNOW LOAD = 70 PSF  
(Reference: 2018 North Carolina Residential Code Section 403.1.1)**

LOAD BEARING VALUE OF SOIL (PSF)			4000								
FOOTING WIDTH, W (IN.)			16	20	24	28	32	36	40	44	48
FOUNDATION WALL SUPPORTING	FOOTING THICKNESS, T (IN.)	WALL LOAD ON FOOTING (PLF)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)	MAXIMUM COLUMN LOAD (LB.)
ROOF AND CEILING OVER SLAB-ON-GRADE (FIGURE 1)	8	2270	16136	23618	30206	37241					
	10	2270	16902	24308	31713	39119	46525				
	12	2270	17646	25417	33188	40958	48729	56500			
	14	2270	18368	26498	34628	42759	50889	59020	67150		
	16	2270	19067	27551	36036	44520	53005	61489	69974	78458	
	18	2270	19744	28577	37410	46243	55076	63909	72742	81575	90408
20	2270	20399	29575	38751	47927	57103	66278	75454	84630	93806	
ROOF, CEILING AND ONE CENTER-BEARING FLOOR (FIGURE 2)	8	2720	13623	20658	27693	34728					
	10	2720	14239	21645	29051	36457	43862				
	12	2720	14833	22604	30375	38146	45917	53688			
	14	2720	15405	23535	31666	39796	47927	56057	64188		
	16	2720	15954	24439	32923	41408	49892	58377	66861	75346	
	18	2720	16482	25315	34148	42980	51813	60646	69479	78312	87145
20	2720	16987	26163	35338	44514	53690	62866	72042	81218	90393	
ROOF, CEILING AND ONE CLEAR-SPAN FLOOR (FIGURE 3)	8	3170	11111	18146	25181	32216					
	10	3170	11577	18983	26388	33794	41200				
	12	3170	12021	19792	27563	35333	43104	50875			
	14	3170	12443	20573	28703	36834	44964	53095	61225		
	16	3170	12842	21326	29811	38295	46780	55264	63749	72233	
	18	3170	13219	22052	30885	39718	48551	57384	66217	75050	83883
20	3170	13574	22750	31926	41102	50278	59453	68629	77805	86981	
ROOF, CEILING AND TWO CENTER-BEARING FLOORS (FIGURE 4)	8	3560	8933	15968	23003	30038					
	10	3560	9269	16675	24081	31487	38892				
	12	3560	9583	17354	25125	32896	40667	48438			
	14	3560	9875	18005	26136	34266	42397	50527	58658		
	16	3560	10144	18629	27113	35598	44082	52567	61051	69536	
	18	3560	10392	19225	28058	36890	45723	54556	63389	72222	81055
20	3560	10617	19793	28968	38144	47320	56496	65672	74848	84023	
ROOF, CEILING AND TWO CLEAR-SPAN FLOORS (FIGURE 5)	8	4370	4411	11446	18481	25516					
	10	4370	4477	11883	19288	26694	34100				
	12	4370	4521	12292	20063	27833	35604	43375			
	14	4370	4543	12673	20803	28934	37064	45195	53325		
	16	4370	4542	13026	21511	29995	38480	46964	55449	63933	
	18	4370	4519	13352	22185	31018	39851	48684	57517	66350	75183
20	4370	4474	13650	22826	32002	41178	50353	59529	68705	77881	
ROOF, CEILING AND THREE CENTER-BEARING FLOORS (FIGURE 6)	8	4400	4243	11278	18313	25348					
	10	4400	4299	11705	19111	26517	33922				
	12	4400	4333	12104	19875	27646	35417	43188			
	14	4400	4345	12475	20606	28736	36867	44997	53128		
	16	4400	4334	12819	21303	29788	38272	46757	55241	63726	
	18	4400	4302	13135	21968	30800	39633	48466	57299	66132	74965
20	4400	4247	13423	22598	31774	40950	50126	59302	68478	77653	
ROOF, CEILING AND THREE CLEAR-SPAN FLOORS (FIGURE 7)	8	5570	-	4746	11781	18816					
	10	5570	-	4783	12188	19594	27000				
	12	5570	-	4792	12563	20333	28104	35875			
	14	5570	-	4773	12903	21034	29164	37295	45425		
	16	5570	-	4726	13211	21695	30180	38664	47149	55633	
	18	5570	-	4652	13485	22318	31151	39984	48817	57650	66483
20	5570	-	4550	13726	22902	32078	41253	50429	59605	68781	

**NOTES:**

1. Refer to Table Notes on page 1.7 for additional information.
2. Refer to Figures 1 through 9 on pages 1.10 and 1.11 for additional information.

## TABLES 1.1 THROUGH 1.6: MAXIMUM COLUMN LOAD ON CONTINUOUS CAST-IN-PLACE CONCRETE FOOTING AT EXTERIOR WALL

### TABLE NOTES:

1. To use Tables 1.1 through 1.6:
  - a. Select the table for the appropriate Load Bearing Value of Soil and exterior wall finish (without brick veneer or with brick veneer).
  - b. Select the row corresponding to the configuration of the structure (number of floors supported on foundation).
  - c. Select a footing width and thickness within that row that has a corresponding maximum column load equal to or greater than the required column load.
2. Tables 1.1 through 1.6 are for use with one- and two-family residential structures constructed in accordance with the 2018 North Carolina Residential Code (2015 International Residential Code with North Carolina amendments).
3. Tables 1.1 through 1.6 are for use with one- and two-family residential structures in Seismic Design Category A, B, or C.
4. Footing sizes indicated in Tables 1.1 through 1.6 have been evaluated for the effects of gravity loads only. The inclusion of lateral loads due to wind or seismic forces in the footing designs is beyond the scope of these tables.
5. Footing sizes in the shaded area of Tables 1.1 through 1.6 are not in compliance with the minimum size requirements of the 2018 North Carolina Residential Code, Section R403.1.1.
6. Maximum column loads and footing sizes indicated in Tables 1.1 through 1.6 are based on the following design criteria and assumptions:
  - a. House is 36-feet wide, both with and without a load-bearing center wall that carries half of the tributary floor framing, as indicated in the table headings.
  - b. House is built over 2-foot crawl space, unless noted otherwise on table.
  - c. House has 10-foot floor-to-floor heights. Tables 1.1 through 1.6 may be used for wall heights less than or equal to 10 feet.
  - d. Foundations are plain cast-in-place concrete footings constructed in accordance with Chapter 4 of the 2018 North Carolina Residential Code.
  - e. Foundation wall consists of minimum 8-inch hollow CMU laid in running bond and is constructed in accordance with the 2018 North Carolina Residential Code.
  - f. Foundation wall is 24-inches high and is centered on the footing, as shown in Figures 8 and 9 on Page 1.11.
  - g. Design loads:
 

i. First Floor	50 psf (40 psf live load, 10 psf dead load)
ii. Second Floor	40 psf (30 psf live load, 10 psf dead load)
iii. Third Floor	40 psf (30 psf live load, 10 psf dead load)
iv. Roof	90 psf (70 psf snow load, 20 psf dead load)
v. Footing (self-weight)	150 pcf (normal weight concrete)
vi. Soil (self-weight)	120 pcf
vii. 8-inch CMU	55 psf
viii. 12-inch CMU	85 psf
ix. Exterior Wood-frame Wall	12 psf
x. Exterior Wood-frame Wall With Brick Veneer	48 psf
  - h. Habitable attics are considered a supported floor.
7. Refer to Figures 1 through 9 on pages 1.10 and 1.11 for additional information.
8. Footing sizes listed in Tables 1.1 through 1.6 with no column load are not adequate for the load bearing value of the soil indicated with the assumptions and design loads listed above taken into consideration.
9. Maximum column loads indicated in Tables 1.1 through 1.6 are based on the requirements for concrete footings in Section R403.1.1 of the 2018 North Carolina Residential Code and the calculated soil bearing pressure with the assumptions listed above taken into consideration. The evaluation of the masonry foundation wall is not included in the tables and the assumed masonry foundation wall described above may not be in compliance with the 2018 North Carolina Residential Code in all cases. The masonry foundation wall must be evaluated for the applied column load(s) as required by the provisions of the 2018 North Carolina Residential Code.

**TABLE 1.7: PIER AND FOOTING SIZES FOR SUPPORT OF INTERIOR GIRDERS**

(Reference: 2018 North Carolina Residential Code Table R403.1(2))

<b>1 (ONE) STORY</b>			
<b>FLOOR AREA SUPPORTED (SQ. FT.)</b>	<b>PIER (W X L)</b>	<b>FOOTING (W X L X T)</b>	<b>TOTAL ALLOWABLE LOAD (LB.)</b>
50	8" x 16"	1'-4" x 2'-0" x 8"	<b>2500</b>
100	8" x 16"	1'-4" x 2'-0" x 8"	<b>5000</b>
150	8" x 16"	2'-0" x 2'-0" x 8"	<b>7500</b>
200	8" x 16"	2'-4" x 2'-4" x 10"	<b>10000</b>
250	-	-	-
300	-	-	-
<b>2 (TWO) STORY</b>			
<b>FLOOR AREA SUPPORTED (SQ. FT.)</b>	<b>PIER (W X L)</b>	<b>FOOTING (W X L X T)</b>	<b>TOTAL ALLOWABLE LOAD (LB.)</b>
50	8" x 16"	1'-4" x 2'-6" x 8"	<b>4500</b>
100	8" x 16"	2'-0" x 2'-0" x 10"	<b>9000</b>
150	16" x 16"	2'-8" x 2'-8" x 10"	<b>13500</b>
200	16" x 16"	3'-0" x 3'-0" x 10"	<b>18000</b>
250	16" x 16"	3'-4" x 3'-4" x 1'-0"	<b>22500</b>
300	16" x 16"	3'-8" x 3'-8" x 1'-0"	<b>27000</b>
<b>3 (THREE) STORY</b>			
<b>FLOOR AREA SUPPORTED (SQ. FT.)</b>	<b>PIER (W X L)</b>	<b>FOOTING (W X L X T)</b>	<b>TOTAL ALLOWABLE LOAD (LB.)</b>
50	8" x 16"	1'-4" x 2'-6" x 8"	<b>6500</b>
100	16" x 16"	2'-6" x 2'-6" x 10"	<b>13000</b>
150	16" x 16"	3'-0" x 3'-0" x 10"	<b>19500</b>
200	16" x 16"	4'-0" x 4'-0" x 1'-0"	<b>26000</b>
250	16" x 24"	4'-0" x 4'-0" x 1'-0"	<b>32500</b>
300	16" x 24"	4'-6" x 4'-6" x 1'-0"	<b>39000</b>

**NOTES:**

1. Refer to Table Notes on page 1.9 for additional information.

## TABLE 1.7: PIER AND FOOTING SIZES FOR SUPPORT OF INTERIOR GIRDERS

### TABLE NOTES:

1. Table 1.7 expands on the data presented in Table 403.1(2) of the 2018 North Carolina Residential Code to include the *Total Allowable Load* that may be applied to the pier.
2. Table 1.7 is for use with one- and two-family residential structures constructed in accordance with the 2018 North Carolina Residential Code (2015 International Residential Code with North Carolina amendments).
3. Table 1.7 is for use with one- and two-family residential structures in Seismic Design Category A, B, or C.
4. Tabulated pier sizes are based on hollow CMU (concrete masonry unit) piers capped with 4 inches of solid masonry or concrete for 1 (one) story houses, 8 inches of solid masonry or concrete for 2 (two), 2½ (two-and-one-half) and 3 (three) story houses, or the cavities of the top course of masonry may be filled with concrete or grout.
5. Piers shall be constructed with Type S mortar.
6. Tabulated pier sizes are minimums. For height/thickness limitations see Section R606.7 of the North Carolina Residential Code.
7. Center of pier shall bear in the middle one-third of the footing.
8. Footing sizes are based on 2000 psf allowable soil bearing pressure and concrete with a minimum specified compression strength of 2500 psi.
9. Footings shall be full thickness over the entire area of the footing.
10. Girders must have full bearing on the piers.
11. The tabulated *Total Allowable Load* values represent the maximum load that may be applied to the corresponding tabulated masonry piers and are based on the preceding notes and the following design criteria and assumptions:
  - a. Structure is constructed with dimensional framing lumber in accordance with the 2018 North Carolina Residential Code.
  - b. Piers are concrete masonry piers constructed in accordance with Section R606 of the North Carolina Residential Code.
  - c. Footings are plain cast-in-place concrete footings constructed in accordance with Chapter 4 of the 2018 North Carolina Residential Code.
  - d. Piers and footings are located at the interior of the structure (not at the exterior walls).
  - e. Piers and footings support only floor loads (not roof loads).
  - f. Tabulated *Floor Area Supported* is the tributary area of each floor level supported by the corresponding pier and footing.
  - g. Design loads:
    - i. First Floor 50 psf (40 psf live load, 10 psf dead load)
    - ii. Second Floor 40 psf (30 psf live load, 10 psf dead load)
    - iii. Third Floor 40 psf (30 psf live load, 10 psf dead load)

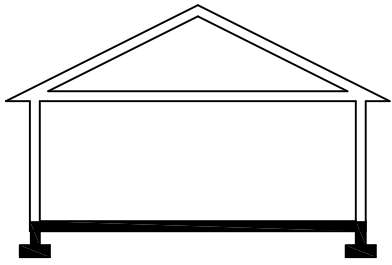


FIGURE 1: ROOF AND CEILING OVER SLAB-ON-GRADE

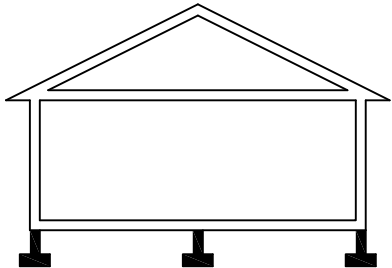


FIGURE 2: ROOF, CEILING AND ONE CENTER-BEARING FLOOR

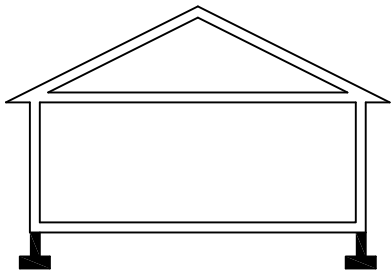


FIGURE 3: ROOF, CEILING AND ONE CLEAR-SPAN FLOOR

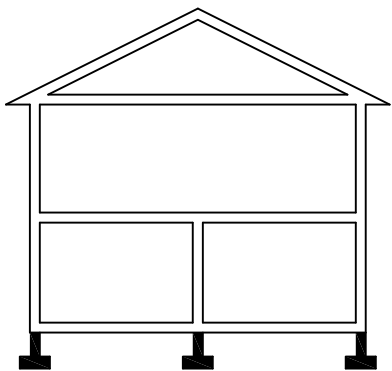


FIGURE 4: ROOF, CEILING AND TWO CENTER-BEARING FLOORS

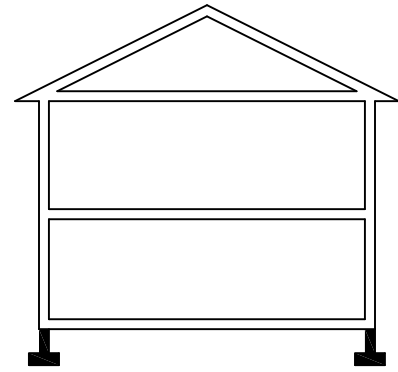


FIGURE 5: ROOF, CEILING AND TWO CLEAR-SPAN FLOORS

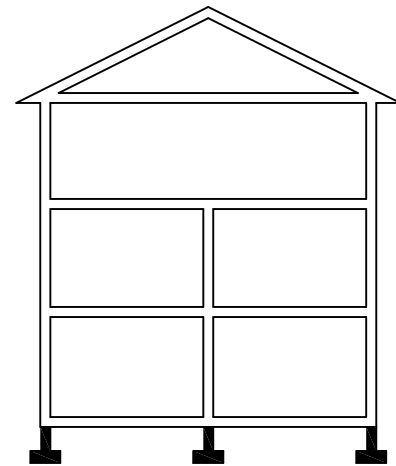


FIGURE 6: ROOF, CEILING AND THREE CENTER-BEARING FLOORS

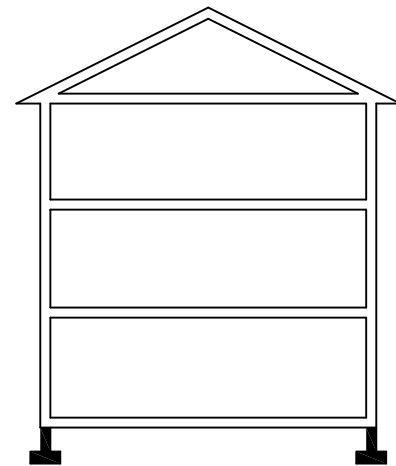
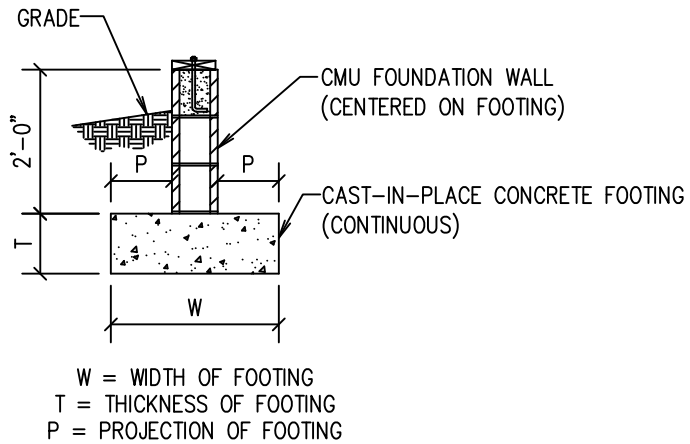
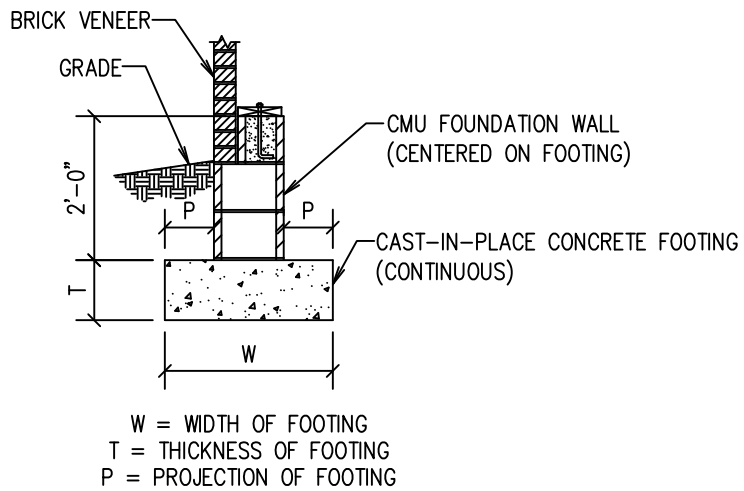


FIGURE 7: ROOF, CEILING AND THREE CLEAR-SPAN FLOORS



FOOTING PROJECTION P SHALL BE NOT LESS THAN 2 INCHES AND SHALL NOT EXCEED FOOTING THICKNESS T

FIGURE 8: CONCRETE FOOTING WITH CMU FOUNDATION WALL FOR LIGHT FRAME CONSTRUCTION



FOOTING PROJECTION P SHALL BE NOT LESS THAN 2 INCHES AND SHALL NOT EXCEED FOOTING THICKNESS T

FIGURE 9: CONCRETE FOOTING WITH CMU FOUNDATION WALL FOR LIGHT FRAME CONSTRUCTION WITH BRICK VENEER



# **JACK STUD TABLES**

**TABLE 2.1: MAXIMUM HEADER / GIRDER END REACTION  
FOR SPECIFIED NUMBER OF JACK STUDS AND HEADER  
THICKNESS**

(Reference: 2018 North Carolina Residential Code Section R502.5 and  
Table R602.7(1))

<i>HEADER THICKNESS</i>	<i>3" (2-2x)</i>	<i>4.5" (3-2X)</i>	<i>6" (4-2x)</i>
<i>NUMBER OF JACK STUDS</i>	<i>MAXIMUM END REACTION (LB.)</i>	<i>MAXIMUM END REACTION (LB.)</i>	<i>MAXIMUM END REACTION (LB.)</i>
<b>1</b>	<b>1660</b>	<b>1760</b>	<b>2760</b>
<b>2</b>	<b>3320</b>	<b>3520</b>	<b>5520</b>
<b>3</b>	<b>4980</b>	<b>5280</b>	<b>8280</b>
<b>4</b>	<b>6640</b>	<b>7040</b>	<b>11040</b>

**NOTES:**

1. Refer to Table Notes on page 2.2 for additional information.

## **TABLE 2.1: MAXIMUM HEADER / GIRDER END REACTION FOR SPECIFIED NUMBER OF JACK STUDS AND HEADER THICKNESS**

### **TABLE NOTES:**

1. Table 2.1 provides the maximum header or girder reaction for a given number of jack studs and a given header or girder thickness (number of nominal 2x plies).
2. Table 2.1 is for use with one- and two-family residential structures constructed in accordance with the 2018 North Carolina Residential Code (2015 International Residential Code with North Carolina amendments).
3. Table 2.1 is for use with one- and two-family residential structures in Seismic Design Category A, B, or C.
4. Jack studs must be a minimum of 2x nominal thickness and have a width at least equal to the width of the wall studs. Wall stud heights shall be in accordance with Table R602.3(5).
5. Tabulated values assume #2 grade lumber and Douglas fir-larch, hem-fir, southern pine and/or spruce-pine-fir lumber species.
6. Tabulated values are derived from the maximum spans and required number of jack studs listed in Tables R602.7(1) and R602.7(2), and the minimum design loads specified in Section R301 of the 2018 North Carolina Residential Code. Refer to Tables 2.4 and 2.5 on pages 2.6 and 2.7 for additional information.
7. Support for header and girder ends must provide a continuous load path from the bearing to the foundation.
8. For header and girder reactions greater than the tabulated values, the structural elements supporting the header or girder must be designed in accordance with accepted engineering practice by a registered design professional.

**TABLE 2.2: LOAD CHART FOR END REACTIONS OF GIRDERS AND HEADERS IN EXTERIOR BEARING WALLS**

GROUND SNOW LOAD = 70 PSF, ROOF SNOW LOAD = 70 PSF

(Reference: 2018 North Carolina Residential Code Section R502.5 and Table R602.7(1))

BUILDING WIDTH = 36 FT.

GIRDERS AND HEADERS SUPPORTING	1st FLOOR SPAN (FT.)	2nd FLOOR SPAN (FT.)	3rd FLOOR SPAN (FT.)	ROOF SPAN (FT.)	HEADER / GIRDER SPAN (FT.)	1st FLOOR LIVE (PSF)	1st FLOOR DEAD (PSF)	2nd FLOOR LIVE (PSF)	2nd FLOOR DEAD (PSF)	3rd FLOOR LIVE (PSF)	3rd FLOOR DEAD (PSF)	ROOF LIVE / SNOW (PSF)	ROOF DEAD (PSF)	WALL LOAD (PLF)	HEADER / GIRDER LOAD (PLF)	HEADER END REACTION (LB.)	NJ		
																	HEADER THICKNESS		
																	3" (2-2x)	4.5" (3-2x)	6" (4-2x)
ROOF AND CEILING (FIGURE 10)	0	0	0	36	2	40	10	30	10	30	10	70	10	0	1600	1600	1	1	1
	0	0	0	36	4	40	10	30	10	30	10	70	10	0	1600	3200	2	2	2
	0	0	0	36	6	40	10	30	10	30	10	70	10	0	1600	4800	3	3	2
	0	0	0	36	8	40	10	30	10	30	10	70	10	0	1600	6400	4	4	3
	0	0	0	36	10	40	10	30	10	30	10	70	10	0	1600	8000			3
	0	0	0	36	12	40	10	30	10	30	10	70	10	0	1600	9600			4
	0	0	0	36	14	40	10	30	10	30	10	70	10	0	1600	11200			
ROOF, CEILING AND ONE CENTER-BEARING FLOOR (FIGURE 11)	0	18	0	36	2	40	10	30	10	30	10	70	10	108	2068	2068	2	2	1
	0	18	0	36	4	40	10	30	10	30	10	70	10	108	2068	4136	3	3	2
	0	18	0	36	6	40	10	30	10	30	10	70	10	108	2068	6204	4	4	3
	0	18	0	36	8	40	10	30	10	30	10	70	10	108	2068	8272			3
	0	18	0	36	10	40	10	30	10	30	10	70	10	108	2068	10340			4
	0	18	0	36	12	40	10	30	10	30	10	70	10	108	2068	12408			
	0	18	0	36	14	40	10	30	10	30	10	70	10	108	2068	14476			
ROOF, CEILING AND ONE CLEAR-SPAN FLOOR (FIGURE 12)	0	36	0	36	2	40	10	30	10	30	10	70	10	108	2428	2428	2	2	1
	0	36	0	36	4	40	10	30	10	30	10	70	10	108	2428	4856	3	3	2
	0	36	0	36	6	40	10	30	10	30	10	70	10	108	2428	7284			3
	0	36	0	36	8	40	10	30	10	30	10	70	10	108	2428	9712			4
	0	36	0	36	10	40	10	30	10	30	10	70	10	108	2428	12140			
	0	36	0	36	12	40	10	30	10	30	10	70	10	108	2428	14568			
	0	36	0	36	14	40	10	30	10	30	10	70	10	108	2428	16996			
ROOF, CEILING AND TWO CENTER-BEARING FLOORS (FIGURE 13)	0	18	18	36	2	40	10	30	10	30	10	70	10	204	2524	2524	2	2	1
	0	18	18	36	4	40	10	30	10	30	10	70	10	204	2524	5048	4	3	2
	0	18	18	36	6	40	10	30	10	30	10	70	10	204	2524	7572			3
	0	18	18	36	8	40	10	30	10	30	10	70	10	204	2524	10096			4
	0	18	18	36	10	40	10	30	10	30	10	70	10	204	2524	12620			
	0	18	18	36	12	40	10	30	10	30	10	70	10	204	2524	15144			
	0	18	18	36	14	40	10	30	10	30	10	70	10	204	2524	17668			
ROOF, CEILING AND TWO CLEAR-SPAN FLOORS (FIGURE 14)	0	36	36	36	2	40	10	30	10	30	10	70	10	204	3244	3244	2	2	2
	0	36	36	36	4	40	10	30	10	30	10	70	10	204	3244	6488	4	4	3
	0	36	36	36	6	40	10	30	10	30	10	70	10	204	3244	9732			4
	0	36	36	36	8	40	10	30	10	30	10	70	10	204	3244	12976			
	0	36	36	36	10	40	10	30	10	30	10	70	10	204	3244	16220			
	0	36	36	36	12	40	10	30	10	30	10	70	10	204	3244	19464			
	0	36	36	36	14	40	10	30	10	30	10	70	10	204	3244	22708			
0	36	36	36	16	40	10	30	10	30	10	70	10	204	3244	25952				

**NOTES:**

1. Refer to Table Notes on page 2.5 for additional information.
2. Refer to Figures 10 through 14 on page 2.9 for additional information.

**TABLE 2.3: LOAD CHART FOR END REACTIONS OF GIRDERS AND HEADERS IN INTERIOR BEARING WALLS**

(Reference: 2018 North Carolina Residential Code Section R502.5 and Table R602.7(2))

BUILDING WIDTH = 36 FT.																			
GIRDERS AND HEADERS SUPPORTING	1st FLOOR SPAN (FT.)	2nd FLOOR SPAN (FT.)	3rd FLOOR SPAN (FT.)	ROOF SPAN (FT.)	HEADER / GIRDER SPAN (FT.)	1st FLOOR LIVE (PSF)	1st FLOOR DEAD (PSF)	2nd FLOOR LIVE (PSF)	2nd FLOOR DEAD (PSF)	3rd FLOOR LIVE (PSF)	3rd FLOOR DEAD (PSF)	ROOF LIVE / SNOW (PSF)	ROOF DEAD (PSF)	WALL LOAD (PLF)	HEADER / GIRDER LOAD (PLF)	END REACTION (L.B.)	NJ		
																	HEADER THICKNESS		
																	3" (2-2x)	4.5" (3-2x)	6" (4-2x)
ONE FLOOR ONLY (FIGURE 15)	18	0	0	0	2	40	10	30	10	30	10	20	10	0	900	900	1	1	1
	18	0	0	0	4	40	10	30	10	30	10	20	10	0	900	1800	2	2	1
	18	0	0	0	6	40	10	30	10	30	10	20	10	0	900	2700	2	2	1
	18	0	0	0	8	40	10	30	10	30	10	20	10	0	900	3600	3	3	2
	18	0	0	0	10	40	10	30	10	30	10	20	10	0	900	4500	3	3	2
	18	0	0	0	12	40	10	30	10	30	10	20	10	0	900	5400	4	4	2
TWO FLOORS (FIGURE 16)	18	0	0	0	14	40	10	30	10	30	10	20	10	0	900	6300	4	4	3
	18	0	0	0	16	40	10	30	10	30	10	20	10	0	900	7200			3
	18	18	0	0	2	40	10	30	10	30	10	20	10	80	1700	1700	2	1	1
	18	18	0	0	4	40	10	30	10	30	10	20	10	80	1700	3400	3	2	2
	18	18	0	0	6	40	10	30	10	30	10	20	10	80	1700	5100	4	3	2
	18	18	0	0	8	40	10	30	10	30	10	20	10	80	1700	6800		4	3
THREE FLOORS (FIGURE 17)	18	18	0	0	10	40	10	30	10	30	10	20	10	80	1700	8500			4
	18	18	0	0	12	40	10	30	10	30	10	20	10	80	1700	10200			4
	18	18	0	0	14	40	10	30	10	30	10	20	10	80	1700	11900			
	18	18	0	0	16	40	10	30	10	30	10	20	10	80	1700	13600			
	18	18	18	0	2	40	10	30	10	30	10	20	10	152	2492	2492	2	2	1
	18	18	18	0	4	40	10	30	10	30	10	20	10	152	2492	4984	4	3	2
THREE FLOORS (FIGURE 17)	18	18	18	0	6	40	10	30	10	30	10	20	10	152	2492	7476			3
	18	18	18	0	8	40	10	30	10	30	10	20	10	152	2492	9968			4
	18	18	18	0	10	40	10	30	10	30	10	20	10	152	2492	12460			
	18	18	18	0	12	40	10	30	10	30	10	20	10	152	2492	14952			
	18	18	18	0	14	40	10	30	10	30	10	20	10	152	2492	17444			
	18	18	18	0	16	40	10	30	10	30	10	20	10	152	2492	19936			

**NOTES:**

1. Refer to Table Notes on page 2.5 for additional information.
2. Refer to Figures 15 through 17 on page 2.10 for additional information.

## TABLES 2.2 AND 2.3: LOAD CHART FOR END REACTIONS OF GIRDERS AND HEADERS IN EXTERIOR AND INTERIOR BEARING WALLS

### TABLE NOTES:

1. Tables 2.2 and 2.3 provide a summary of header/girder loads and reactions for several header/girder spans and loading conditions. Tables 2.2 and 2.3 also provide the number of jack studs required to support each end of the header/girder for each of the spans and loading conditions.
2. Tables 2.2 and 2.3 are for use with one- and two-family residential structures constructed in accordance with the 2018 North Carolina Residential Code (2015 International Residential Code with North Carolina amendments).
3. Tables 2.2 and 2.3 are for use with one- and two-family residential structures in Seismic Design Category A, B, or C.
4. NJ = Number of jack studs required to support each end of the header/girder.
5. Jack studs must be a minimum of 2x nominal thickness and have a width at least equal to the width of the wall studs. Wall stud heights shall be in accordance with Table R602.3(5).
6. Tabulated values assume #2 grade lumber and Douglas fir-larch, hem-fir, southern pine and/or spruce-pine-fir lumber species.
7. Tabulated values are derived from the maximum spans and required number of jack studs listed in Tables R602.7(1) and R602.7(2), and the minimum design loads specified in Section R301 of the 2018 North Carolina Residential Code. Refer to Table 2.1 on page 2.1 and Tables 2.4 and 2.5 on pages 2.6 and 2.7 for additional information.
8. The header/girder reactions and the number of jack studs required to support the headers/girders indicated in Tables 2.2 and 2.3 are based on the following design criteria and assumptions:
  - a. 36-foot wide house with 2-foot roof overhang, both with and without a load-bearing center wall that carries half of the tributary floor framing, as indicated in the table headings.
  - b. Story heights:
 

i. First Story	10 feet
ii. Second Story	9 feet
iii. Third Story	8 feet
  - c. Design loads:
 

i. First Floor	50 psf (40 psf live load, 10 psf dead load)
ii. Second Floor	40 psf (30 psf live load, 10 psf dead load)
iii. Third Floor	40 psf (30 psf live load, 10 psf dead load)
iv. Roof	80 psf (70 psf snow load, 10 psf dead load)
v. Exterior Wood-frame Wall	12 psf
vi. Interior Wood-frame Wall	8 psf
  - d. Habitable attics are considered a supported floor.
  - e. Headers and girders in interior bearing walls support only floor loads (not roof loads).
9. Refer to Figures 10 through 17 on pages 2.9 and 2.10 for additional information.
10. Support for header and girder ends must provide a continuous load path from the bearing to the foundation.
11. Where the number of jack studs required to support the header/girder is not listed on Tables 2.2 and 2.3 (shaded areas of table), the structural elements supporting the header/girder must be designed in accordance with accepted engineering practice by a registered design professional.

**TABLE 2.4: REACTIONS OF GIRDERS AND HEADERS IN EXTERIOR BEARING WALLS**

**GROUND SNOW LOAD = 70 PSF, ROOF SNOW LOAD = 70 PSF**

**(Reference: 2018 North Carolina Residential Code Section R502.5 and Table R602.7(1))**

GIRDERS AND HEADERS SUPPORTING	GIRDER / HEADER SIZE	BUILDING WIDTH = 20 FT.			BUILDING WIDTH = 28 FT.			BUILDING WIDTH = 36 FT.		
		SPAN (FT.)	END REACTION (LB.)	NJ	SPAN (FT.)	END REACTION (LB.)	NJ	SPAN (FT.)	END REACTION (LB.)	NJ
ROOF AND CEILING (FIGURE 10)	2 - 2x4	2.833	1360	1	2.500	1600	1	2.250	1800	1
	2 - 2x6	4.167	2000	1	3.667	2347	2	3.250	2600	2
	2 - 2x8	5.333	2560	2	4.583	2933	2	4.083	3266	2
	2 - 2x10	6.500	3120	2	5.583	3573	2	5.000	4000	2
	2 - 2x12	7.500	3600	2	6.500	4160	2	5.833	4666	3
	3 - 2x8	6.670	3202	1	5.750	3680	2	5.167	4134	2
	3 - 2x10	8.167	3920	2	7.000	4480	2	6.333	5066	2
	3 - 2x12	9.417	4520	2	8.167	5227	2	7.333	5866	2
	4 - 2x8	7.667	3680	1	6.667	4267	1	5.917	4734	2
4 - 2x10	9.417	4520	2	8.167	5227	2	7.250	5800	2	
4 - 2x12	10.917	5240	2	9.417	6027	2	8.417	6734	2	
ROOF, CEILING AND ONE CENTER-BEARING FLOOR (FIGURE 11)	2 - 2x4	2.583	1638	1	2.250	1877	1	2.000	2068	1
	2 - 2x6	3.750	2378	2	3.250	2711	2	2.917	3016	2
	2 - 2x8	4.750	3012	2	4.167	3475	2	3.750	3878	2
	2 - 2x10	5.750	3646	2	5.083	4239	2	4.583	4739	3
	2 - 2x12	6.667	4227	2	5.833	4865	3	5.250	5429	3
	3 - 2x8	5.917	3751	2	5.167	4309	2	4.667	4826	2
	3 - 2x10	7.250	4597	2	6.333	5282	2	5.667	5860	2
	3 - 2x12	8.417	5336	2	7.333	6116	2	6.583	6807	2
	4 - 2x8	6.833	4332	1	6.000	5004	2	5.417	5601	2
4 - 2x10	8.333	5283	2	7.333	6116	2	6.583	6807	2	
4 - 2x12	9.667	6129	2	8.500	7089	2	7.583	7841	2	
ROOF, CEILING AND ONE CLEAR-SPAN FLOOR (FIGURE 12)	2 - 2x4	2.417	1774	1	2.083	2029	1	1.833	2225	1
	2 - 2x6	3.500	2569	2	3.083	3003	2	2.750	3339	2
	2 - 2x8	4.500	3303	2	3.917	3815	2	3.500	4249	2
	2 - 2x10	5.500	4037	2	4.750	4627	2	4.250	5160	3
	2 - 2x12	6.333	4648	2	5.500	5357	3	5.000	6070	3
	3 - 2x8	5.583	4098	2	4.917	4789	2	4.417	5362	2
	3 - 2x10	6.833	5015	2	6.000	5844	2	5.333	6474	2
	3 - 2x12	7.917	5811	2	6.917	6737	2	6.250	7588	2
	4 - 2x8	6.500	4771	1	5.667	5520	2	5.083	6171	2
4 - 2x10	7.917	5811	2	6.917	6737	2	6.167	7487	2	
4 - 2x12	9.167	6729	2	8.000	7792	2	7.167	8701	2	
ROOF, CEILING AND TWO CENTER-BEARING FLOORS (FIGURE 13)	2 - 2x4	2.333	1824	1	2.000	2044	1	1.750	2209	1
	2 - 2x6	3.417	2672	2	3.000	3066	2	2.667	3366	2
	2 - 2x8	4.333	3388	2	3.750	3833	2	3.417	4312	2
	2 - 2x10	5.250	4106	2	4.583	4684	3	4.167	5259	3
	2 - 2x12	6.083	4757	3	5.333	5450	3	4.833	6099	3
	3 - 2x8	5.417	4236	2	4.750	4855	2	4.250	5364	2
	3 - 2x10	6.583	5148	2	5.750	5877	2	5.250	6626	2
	3 - 2x12	7.667	5996	2	6.750	6899	2	6.083	7677	3
	4 - 2x8	6.250	4888	2	5.500	5621	2	4.917	6205	2
4 - 2x10	7.583	5930	2	6.667	6814	2	6.000	7572	2	
4 - 2x12	8.833	6907	2	7.750	7921	2	7.000	8834	2	
ROOF, CEILING AND TWO CLEAR-SPAN FLOORS (FIGURE 14)	2 - 2x4	2.000	1964	1	1.667	2170	1	1.417	2298	2
	2 - 2x6	2.917	2864	2	2.583	3363	2	2.250	3650	2
	2 - 2x8	3.750	3683	2	3.250	4232	2	2.917	4731	3
	2 - 2x10	4.583	4501	3	4.000	5208	3	3.500	5677	3
	2 - 2x12	5.333	5237	3	4.583	5967	3	4.083	6623	4
	3 - 2x8	4.667	4583	2	4.083	5316	2	3.667	5948	2
	3 - 2x10	5.750	5647	2	4.917	6402	2	4.417	7164	3
	3 - 2x12	6.667	6547	2	5.750	7487	3	5.167	8381	3
	4 - 2x8	5.417	5319	2	4.667	6076	2	4.167	6759	2
4 - 2x10	6.583	6465	2	5.750	7487	2	5.083	8245	2	
4 - 2x12	7.667	7529	2	6.667	8680	2	5.917	9597	3	

**NOTES:**

1. Refer to Table Notes on page 2.8 for additional information.
2. Refer to Figures 10 through 14 on page 2.9 for additional information.

**TABLE 2.5: REACTIONS OF GIRDERS AND HEADERS IN INTERIOR BEARING WALLS**

(Reference: 2018 North Carolina Residential Code Section R502.5 and Table R602.7(2))

GIRDERS AND HEADERS SUPPORTING	GIRDER / HEADER SIZE	BUILDING WIDTH = 20 FT.			BUILDING WIDTH = 28 FT.			BUILDING WIDTH = 36 FT.		
		SPAN (FT.)	END REACTION (LB.)	NJ	SPAN (FT.)	END REACTION (LB.)	NJ	SPAN (FT.)	END REACTION (LB.)	NJ
ONE FLOOR ONLY (FIGURE 15)	2 - 2x4	3.083	771	1	2.667	933	1	2.417	1088	1
	2 - 2x6	4.500	1125	1	3.917	1371	1	3.500	1575	1
	2 - 2x8	5.750	1438	1	5.000	1750	2	4.417	1988	2
	2 - 2x10	7.000	1750	2	6.083	2129	2	5.417	2438	2
	2 - 2x12	8.083	2021	2	7.000	2450	2	6.250	2813	2
	3 - 2x8	7.167	1792	1	6.250	2188	1	5.583	2512	2
	3 - 2x10	8.750	2188	1	7.583	2654	2	6.750	3038	2
	3 - 2x12	10.167	2542	2	8.833	3092	2	7.833	3525	2
	4 - 2x8	9.000	2250	1	7.667	2683	1	6.750	3038	1
4 - 2x10	10.083	2521	1	8.750	3063	1	7.833	3525	2	
4 - 2x12	11.750	2938	1	10.167	3558	2	9.083	4087	2	
TWO FLOORS (FIGURE 16)	2 - 2x4	2.167	1062	1	1.833	1228	1	1.583	1346	1
	2 - 2x6	3.167	1552	2	2.750	1843	2	2.417	2054	2
	2 - 2x8	4.083	2001	2	3.500	2345	2	3.167	2692	2
	2 - 2x10	4.917	2409	2	4.250	2848	2	3.833	3258	3
	2 - 2x12	5.750	2818	2	5.000	3350	3	4.417	3754	3
	3 - 2x8	5.083	2491	2	4.417	2959	2	3.917	3329	2
	3 - 2x10	6.167	3022	2	5.333	3573	2	4.833	4108	2
	3 - 2x12	7.167	3512	2	6.250	4188	2	5.417	4604	3
	4 - 2x8	6.083	2981	1	5.250	3518	2	4.667	3967	2
4 - 2x10	7.167	3512	2	6.167	4132	2	5.500	4675	2	
4 - 2x12	8.333	4083	2	7.167	4802	2	6.417	5454	2	

**NOTES:**

1. Refer to Table Notes on page 2.8 for additional information.
2. Refer to Figures 15 through 17 on page 2.10 for additional information.



## TABLES 2.4 AND 2.5: REACTIONS OF GIRDERS AND HEADERS IN EXTERIOR AND INTERIOR BEARING WALLS

### TABLE NOTES:

1. Tables 2.4 and 2.5 expand on the data presented in Table R602.7(1) and Table R602.7(2) of the 2018 North Carolina Residential Code to include the reactions of the headers/girders listed in the tables. The tabulated reactions are based on the design criteria and assumptions described in the following notes.
2. Tables 2.4 and 2.5 are for use with one- and two-family residential structures constructed in accordance with the 2018 North Carolina Residential Code (2015 International Residential Code with North Carolina amendments).
3. Tables 2.4 and 2.5 are for use with one- and two-family residential structures in Seismic Design Category A, B, or C.
4. NJ = Number of jack studs required to support each end of the header/girder.
5. Jack studs must be a minimum of 2x nominal thickness and have a width at least equal to the width of the wall studs. Wall stud heights shall be in accordance with Table R602.3(5).
6. Tabulated values assume #2 grade lumber and Douglas fir-larch, hem-fir, southern pine and/or spruce-pine-fir lumber species.
7. Tabulated reactions are derived from the minimum design loads specified in Section R301 of the 2018 North Carolina Residential Code.
8. The header/girder reactions tabulated in Tables 2.4 and 2.5 are based on the following design criteria and assumptions:
  - a. House with 2-foot roof overhang, both with and without a load-bearing center wall that carries half of the tributary floor framing, as indicated in the table headings. Width of house as indicated in the table headings.
  - b. Story heights:
 

i. First Story	10 feet
ii. Second Story	9 feet
iii. Third Story	8 feet
  - c. Design loads:
 

i. First Floor	50 psf (40 psf live load, 10 psf dead load)
ii. Second Floor	40 psf (30 psf live load, 10 psf dead load)
iii. Third Floor	40 psf (30 psf live load, 10 psf dead load)
iv. Roof	80 psf (70 psf snow load, 10 psf dead load)
v. Exterior Wood-frame Wall	12 psf
vi. Interior Wood-frame Wall	8 psf
  - d. Habitable attics are considered a supported floor.
  - e. Headers and girders in interior bearing walls support only floor loads (not roof loads).
9. Refer to Figures 10 through 17 on pages 2.9 and 2.10 for additional information.

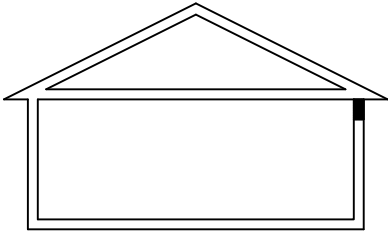


FIGURE 10: ROOF AND CEILING

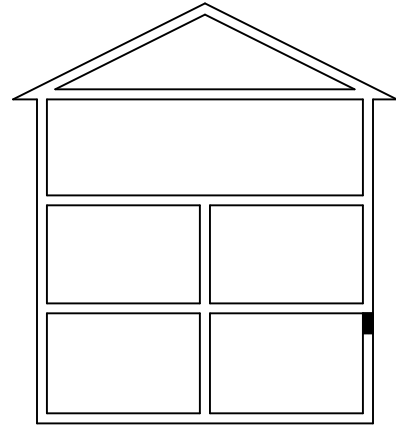


FIGURE 13: ROOF, CEILING AND TWO CENTER-BEARING FLOORS

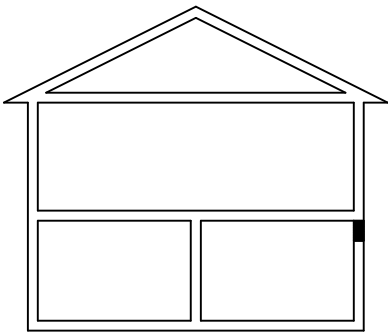


FIGURE 11: ROOF, CEILING AND ONE CENTER-BEARING FLOOR

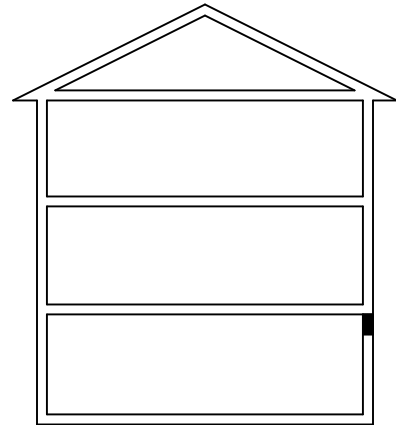


FIGURE 14: ROOF, CEILING AND TWO CLEAR-SPAN FLOORS

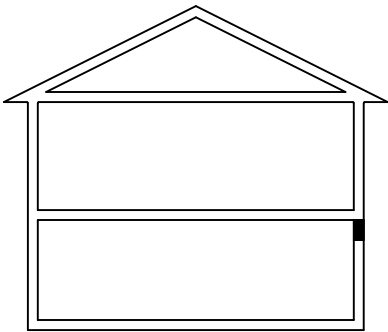


FIGURE 12: ROOF, CEILING AND ONE CLEAR-SPAN FLOOR

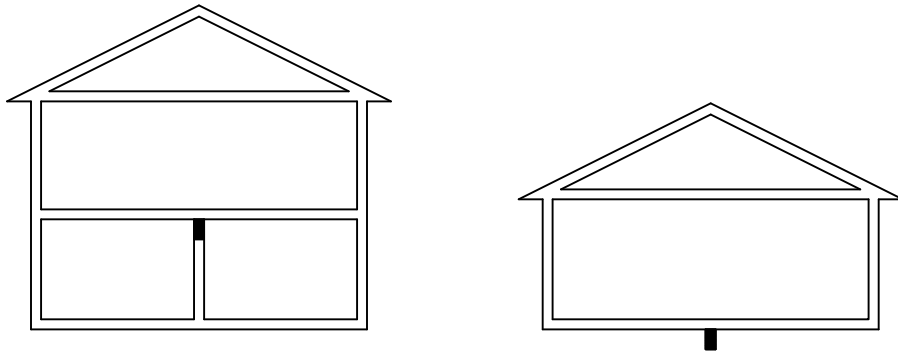


FIGURE 15: ONE FLOOR ONLY

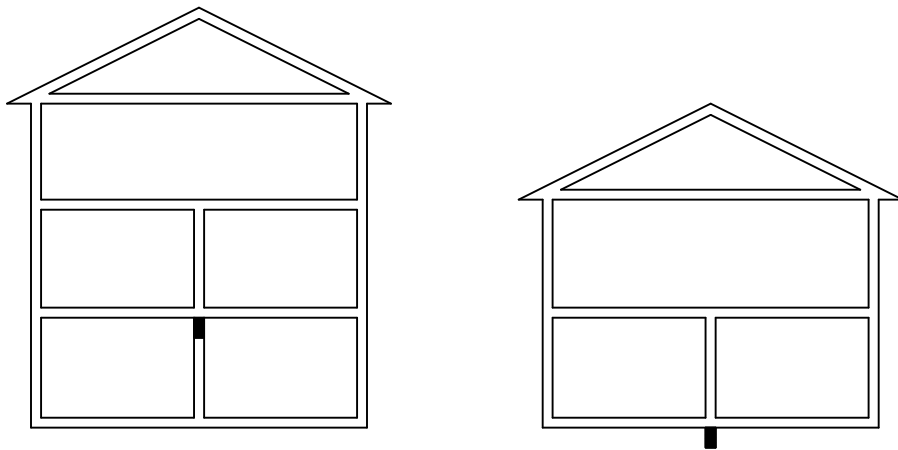


FIGURE 16: TWO FLOORS

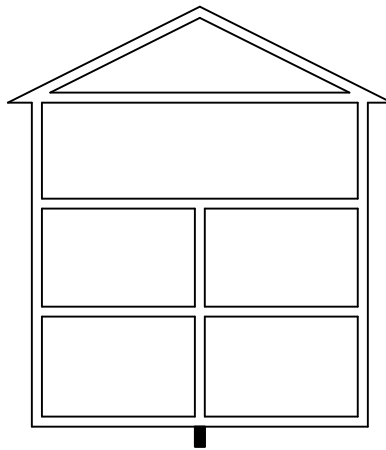


FIGURE 17: THREE FLOORS

# **BEAM TABLES**

**TABLE 3.1: WOOD BEAMS AND GIRDERS ALLOWABLE LOADS AND CORRESPONDING REACTIONS**

(Reference: 2018 North Carolina Residential Code Table W-1)

2x8 (1-1/2" x 7-1/4")												
BEAM / GIRDER SPAN, L (FT.)	SPRUCE-PINE-FIR						SOUTHERN PINE					
	2-PLY		3-PLY		4-PLY		2-PLY		3-PLY		4-PLY	
	LOAD (PLF)	END REACTION (LB.)	LOAD (PLF)	END REACTION (LB.)	LOAD (PLF)	END REACTION (LB.)	LOAD (PLF)	END REACTION (LB.)	LOAD (PLF)	END REACTION (LB.)	LOAD (PLF)	END REACTION (LB.)
3	1305	1958	1956	2934	2610	3915	1692	2538	2538	3807	3383	5075
4	979	1958	1468	2936	1958	3916	1013	2026	1519	3038	2026	4052
5	736	1840	1104	2760	1472	3680	648	1620	972	2430	1296	3240
6	511	1533	767	2301	1022	3066	450	1350	675	2025	900	2700
7	375	1313	563	1971	751	2629	331	1159	496	1736	661	2314
8	287	1148	431	1724	575	2300	253	1012	380	1520	506	2024
9	227	1022	341	1535	454	2043	200	900	300	1350	400	1800
10	184	920	276	1380	368	1840	162	810	243	1215	324	1620
12	114	684	172	1032	228	1368	113	678	169	1014	225	1350
14	72	504	108	756	144	1008	72	504	108	756	144	1008
2x10 (1-1/2" x 9-1/4")												
BEAM / GIRDER SPAN, L (FT.)	SPRUCE-PINE-FIR						SOUTHERN PINE					
	2-PLY		3-PLY		4-PLY		2-PLY		3-PLY		4-PLY	
	LOAD (PLF)	END REACTION (LB.)	LOAD (PLF)	END REACTION (LB.)	LOAD (PLF)	END REACTION (LB.)	LOAD (PLF)	END REACTION (LB.)	LOAD (PLF)	END REACTION (LB.)	LOAD (PLF)	END REACTION (LB.)
3	1665	2498	2498	3747	3330	4995	2158	3237	3238	4857	4317	6476
4	1249	2498	1873	3746	2498	4996	1426	2852	2139	4278	2852	5704
5	999	2498	1499	3748	1998	4995	913	2283	1369	3423	1825	4563
6	763	2289	1144	3432	1525	4575	634	1902	951	2853	1268	3804
7	560	1960	840	2940	1120	3920	466	1631	698	2443	931	3259
8	429	1716	643	2572	858	3432	357	1428	535	2140	713	2852
9	339	1526	508	2286	678	3051	282	1269	423	1904	563	2534
10	275	1375	412	2060	549	2745	228	1140	342	1710	456	2280
12	191	1146	286	1716	381	2286	158	948	238	1428	317	1902
14	140	980	210	1470	280	1960	116	812	175	1225	233	1631
2x12 (1-1/2" x 11-1/4")												
BEAM / GIRDER SPAN, L (FT.)	SPRUCE-PINE-FIR						SOUTHERN PINE					
	2-PLY		3-PLY		4-PLY		2-PLY		3-PLY		4-PLY	
	LOAD (PLF)	END REACTION (LB.)	LOAD (PLF)	END REACTION (LB.)	LOAD (PLF)	END REACTION (LB.)	LOAD (PLF)	END REACTION (LB.)	LOAD (PLF)	END REACTION (LB.)	LOAD (PLF)	END REACTION (LB.)
3	2025	3038	3038	4557	4050	6075	2625	3938	3938	5907	5250	7875
4	1519	3038	2278	4556	3038	6076	1969	3938	2953	5906	3938	7876
5	1215	3038	1823	4558	2430	6075	1266	3165	1898	4745	2531	6328
6	1013	3039	1519	4557	2025	6075	879	2637	1318	3954	1756	5268
7	753	2636	1130	3955	1507	5275	646	2261	969	3392	1291	4519
8	577	2308	856	3424	1154	4616	494	1976	742	2968	989	3956
9	456	2052	684	3078	911	4100	391	1760	586	2637	781	3515
10	369	1845	554	2770	738	3690	316	1580	475	2375	633	3165
12	256	1536	385	2310	513	3078	220	1320	330	1980	439	2634
14	188	1316	283	1981	377	2639	161	1127	242	1694	323	2261

**NOTES:**

1. Refer to Table Notes on page 3.2 for additional information.

## **TABLE 3.1: WOOD BEAMS AND GIRDERS ALLOWABLE LOADS AND CORRESPONDING REACTIONS**

### **TABLE NOTES:**

1. Table 3.1 expands on the data presented in Table W-1 of the 2018 North Carolina Residential Code to include the reactions of the beams and girders for the spans and loads listed in the table. The tabulated reactions were determined by multiplying the span by the corresponding allowable load and dividing by 2.
2. Table 3.1 is for use with one- and two-family residential structures constructed in accordance with the 2018 North Carolina Residential Code (2015 International Residential Code with North Carolina amendments).
3. Table 3.1 is for use with one- and two-family residential structures in Seismic Design Category A, B, or C.
4. Lumber grade is #2 intended for an in-service moisture content of 19% or less.
5. Deflection is limited to  $L/360$ .
6. Load duration factor used in calculations is 1.0.
7. Adequate bearing and lateral support for the member must be provided. Support for the member ends must provide a continuous load path from the bearing to the foundation.
8. Values tabulated are for spruce-pine-fir, not spruce-pine-fir (south). Values tabulated for southern pine are based on design values published by the American Wood Council in an addendum to NDS dated March 2013.
9. Span,  $L$ , is clear span. Effective span for bending and deflection is clear span plus 3 inches.
10. For beam and girder loads and/or spans greater than the tabulated values, the beam or girder must be designed in accordance with accepted engineering practice by a registered design professional.

**TABLE 3.2: FLITCH PLATE BEAMS ALLOWABLE LOADS AND CORRESPONDING REACTIONS**  
 (Reference: 2018 North Carolina Residential Code Table W-2)

<b>(2) 2x6 WITH PLATE INDICATED</b>										
<b>PLATE SIZE / (BEAM WEIGHT PER FOOT)</b>										
<b>BEAM SPAN, L (FT.)</b>	<b>1/4" x 5" PLATE (8 PLF)</b>		<b>3/8" x 5" PLATE (10 PLF)</b>		<b>1/2" x 5" PLATE (13 PLF)</b>		<b>5/8" x 5" PLATE (15 PLF)</b>		<b>3/4" x 5" PLATE (17 PLF)</b>	
	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>
6	643	1929	825	2475	1006	3018	1188	3564	1370	4110
7	473	1656	606	2121	739	2587	873	3056	1006	3521
8	362	1448	464	1856	566	2264	668	2672	771	3084
9	272	1224	348	1566	425	1913	502	2259	579	2606
10	198	990	254	1270	310	1550	366	1830	422	2110
11	149	820	191	1051	233	1282	275	1513	317	1744
12	115	690	147	882	179	1074	212	1272	244	1464
<b>(2) 2x8 WITH PLATE INDICATED</b>										
<b>PLATE SIZE / (BEAM WEIGHT PER FOOT)</b>										
<b>BEAM SPAN, L (FT.)</b>	<b>1/4" x 7" PLATE (11 PLF)</b>		<b>3/8" x 7" PLATE (14 PLF)</b>		<b>1/2" x 7" PLATE (17 PLF)</b>		<b>5/8" x 7" PLATE (20 PLF)</b>		<b>3/4" x 7" PLATE (23 PLF)</b>	
	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>
6	1150	3450	1499	4497	1849	5547	2199	6597	2549	7647
7	845	2958	1102	3857	1359	4757	1615	5653	1872	6552
8	647	2588	843	3372	1040	4160	1237	4948	1434	5736
9	511	2300	666	2997	822	3699	977	4397	1133	5099
10	414	2070	540	2700	666	3330	792	3960	917	4585
11	342	4372	446	6556	550	8743	654	4991	758	7487
12	287	3996	375	5996	462	7992	550	3652	637	5476
13	230	3434	300	5148	369	6863	439	2853	509	4280
14	184	2800	240	4200	296	5600	352	2330	408	3490
15	150	1125	195	1463	240	1800	286	2145	331	2483
16	123	984	161	1288	198	1584	236	1888	273	2184
<b>(2) 2x10 WITH PLATE INDICATED</b>										
<b>PLATE SIZE / (BEAM WEIGHT PER FOOT)</b>										
<b>BEAM SPAN, L (FT.)</b>	<b>1/4" x 9" PLATE (14 PLF)</b>		<b>3/8" x 9" PLATE (18 PLF)</b>		<b>1/2" x 9" PLATE (22 PLF)</b>		<b>5/8" x 9" PLATE (26 PLF)</b>		<b>3/4" x 9" PLATE (30 PLF)</b>	
	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>
6	1642	4926	2145	6435	2649	7947	3153	9459	3657	10971
7	1206	4221	1576	5516	1946	6811	2317	8110	2687	9405
8	923	3692	1207	4828	1490	5960	1774	7096	2057	8228
9	730	3285	954	4293	1177	5297	1401	6305	1625	7313
10	591	2955	772	3860	954	4770	1135	5675	1317	6585
11	488	6075	638	9114	788	12150	938	7875	1088	11814
12	410	5317	536	7973	662	10633	788	6892	914	10336
13	350	4860	457	7292	564	9720	672	5064	779	7592
14	302	4559	394	6836	487	9113	579	3956	672	5931
15	263	3765	343	5650	424	7535	504	3230	585	4845
16	231	6075	302	9114	373	12150	443	7875	514	11814
17	204	5317	267	7973	330	10633	393	6892	456	10336
18	182	4860	238	7292	294	9720	350	5064	406	7592
19	155	1473	203	1929	250	2375	298	2831	345	3278
20	133	1330	174	1740	214	2140	255	2550	296	2960
<b>(2) 2x12 WITH PLATE INDICATED</b>										
<b>PLATE SIZE / (BEAM WEIGHT PER FOOT)</b>										
<b>BEAM SPAN, L (FT.)</b>	<b>1/4" x 11" PLATE (18 PLF)</b>		<b>3/8" x 11" PLATE (22 PLF)</b>		<b>1/2" x 11" PLATE (27 PLF)</b>		<b>5/8" x 11" PLATE (32 PLF)</b>		<b>3/4" x 11" PLATE (36 PLF)</b>	
	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>	<b>LOAD (PLF)</b>	<b>END REACTION (LB.)</b>
6	2297	6891	3006	9018	3715	11145	4425	13275	5134	15402
7	1688	5908	2209	7732	2730	9555	3251	11379	3772	13202
8	1292	5168	1691	6764	2090	8360	2489	9956	2888	11552
9	1021	4595	1336	6012	1651	7430	1966	8847	2282	10269
10	827	4135	1082	5410	1338	6690	1593	7965	1848	9240
11	683	6075	894	9114	1105	12150	1316	7875	1527	11814
12	574	5317	752	7973	929	10633	1106	6892	1283	10336
13	489	4860	640	7292	791	9720	943	5064	1094	7592
14	422	4559	552	6836	682	9113	813	3956	943	5931
15	367	3765	481	5650	594	7535	708	3230	821	4845
16	323	2584	423	3384	522	4176	622	4976	722	5776
17	286	2431	374	3179	463	3936	551	4684	639	5432
18	255	2295	334	3006	413	3717	492	4428	570	5130
19	229	6075	300	9114	371	12150	441	7875	512	11814
20	207	5317	271	7973	334	10633	398	6892	462	10336
21	188	4860	245	7292	303	9720	361	5064	419	7592
22	171	4559	224	6836	276	9113	329	3956	382	5931
23	156	3765	205	5650	253	7535	301	3230	349	4845
24	140	1680	183	2196	226	2712	269	3228	312	3744

**NOTES:**

1. Refer to Table Notes on page 3.4 for additional information.

### **TABLE 3.2: FLITCH PLATE BEAMS ALLOWABLE LOADS AND CORRESPONDING REACTIONS**

#### **TABLE NOTES:**

1. Table 3.2 expands on the data presented in Table W-2 of the 2018 North Carolina Residential Code to include the reactions of the beams for the spans and loads listed in the table. The tabulated reactions were determined by multiplying the span by the corresponding allowable load and dividing by 2.
2. Table 3.2 is for use with one- and two-family residential structures constructed in accordance with the 2018 North Carolina Residential Code (2015 International Residential Code with North Carolina amendments).
3. Table 3.2 is for use with one- and two-family residential structures in Seismic Design Category A, B, or C.
4. Lumber species and grade is #2 southern pine or #2 spruce-pine-fir intended for an in-service moisture content of 19% or less.
5. Southern pine lumber design values were published by the American Wood Council in an addendum to NDS dated March 2013.
6. Tabulated values are based on ASTM A36 structural steel plate.
7. Deflection is limited to  $L/360$ .
8. Load duration factor used in calculations is 1.0.
9. Adequate bearing and lateral support for the member must be provided. Support for the member ends must provide a continuous load path from the bearing to the foundation.
10. Wood side plates and steel flitch plates shall be continuous throughout the span.
11. Span,  $L$ , is center-to-center of supports.
12. For beam and girder loads and/or spans greater than the tabulated values, the beam or girder must be designed in accordance with accepted engineering practice by a registered design professional.